Anne T Antonius

22656 Peach Tree Lane Rocky River, Ohio 44116

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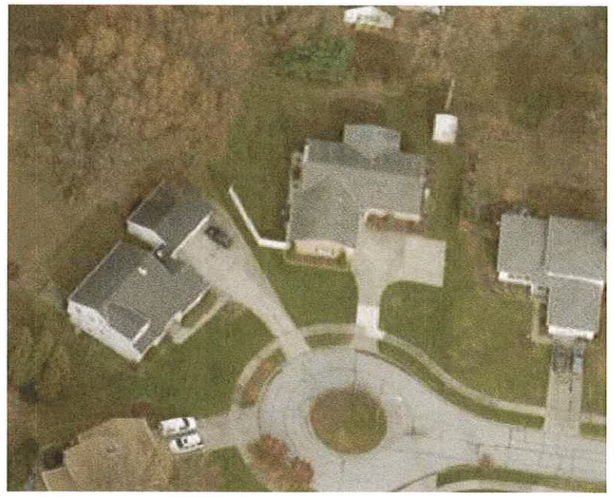
Structural Loading AS APPLICABLE

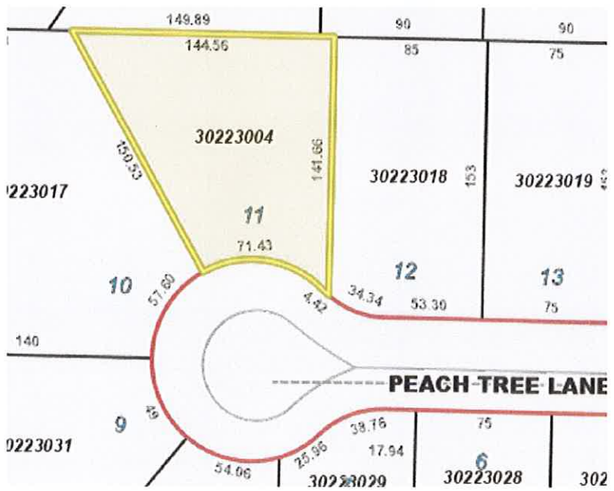
1. ALL STRUCTURAL WOOD MEMBERS SHALL BE STRUCT. GRADE DOUGLAR FIR WITH MIN. Fb 1500 P.S.F. AND LIVE LOAD LISTED BELOW:
   - ATTIC SPACE & ROOF - 30 P.S.F.
   - UNINHABITAL ATTIC, SOFFITS, ECT. - 20 P.S.F.
2. ROOF SNOW LOADS
   - a. Ground Snow Load (Pg) - 20 P.S.F.
   - b. Exposure Factor (Ce) - 1.0
   - c. Importance Factor (I) - 1.0
   - d. Flat Roof Snow Load - N.A.
   - e. Thermal Factor (Ct) - 1.0
3. ROOF LOADS
   - a. Live Load - 20 P.S.F.
   - a. Dead Load - 20 P.S.F.
4. WIND LOADS
   - a. Basic Wind Speed (# Second Gust) - 115 M.P.H.
   - b. Importance Factor - 1.0
   - c. Exposure Category (M-IFRS) - B
   - d. Internal Pressure Coffident - 0.18
5. SEISMIC LOADS
   - a. Seismic Design Category - B
   - b. Seismic Importance Factor - 1.0
   - c. Site Class - D
   - d. Mapped Spectreal Response Accel.
     - S_s = 0.164
     - S_1 = 0.151
   - e. Spectreal Response Coefficient
     - Short Periods (SDS) - 0.175
     - 1 Second Periods (SD1) - 0.061
   - f. Wood Shear Walls Response MOD Factor (R) - 6.5
   - g. Design Base Shear (V) - 2.8k
6. ANBALYSIS BY SIMPLIFIED PROCEDURE
7. ALL PRE-ENGINEERED TRUSSES MUST BE DESIGNED/ STAMPED BY REGISTAR STRUCTURAL ENGINEERED WITH THE STATE OF OHIO SEAL.
8. CONCRETE FLOOR - 100 P.S.F.

GENERAL NOTES:

1. CONTRACTOR SHALL VERIFY EXISTING CONDITIONS PRIOR TO START OF CONSTRUCTION AND ADJUST ALL WORK TO MATCH CONDITIONS ENCOUNTERED.
2. ALL STRUCTURAL WOOD MEMBERS SHALL BE STRUCT. GRADE DOUGLAR FIR WITH MIN. Fb 1500 P.S.F. AND LIVE LOAD LISTED BELOW:
   - FIRST & SECOND FLOORS - 40 P.S.F.
   - ATTIC SPACE & ROOF - 30 P.S.F.
   - UNINHABITAL ATTIC, SOFFITS, ECT. - 20 P.S.F.
3. G.C. MUST COORDINATE ALL M.E.P. ITEMS LOCATED IN WALLS WITH THE RESPECTIVE TRADE. G.C. SHALL CONSTRUCT OR CUT AS REQUIRED OPENINGS AND PROVIDE LINTELS. OPENING SIZES/LOCATIONS SHALL BE PER THE RESPECTIVE TRADE CONTRACTOR.
4. G.C. SHALL CONSTRUCT OR CUT ALL OPENINGS THRU WALLS WHEREVER INDICATED BY ASSOCIATED M.E.P. CONTRACTOR. ALL OPENINGS OVER 14" IN WIDTH SHALL HAVE LINTELS IN ACCORDANCE WITH THE LOOSE LINTEL SCHEDULE.
5. OPENINGS UNDER 14" IN WIDTH WILL NOT REQUIRE LINTELS UNLESS THE STRUCTURAL INTEGRITY OF THE CONSTRUCTION IS AFFECTED. CONTRACTOR NOTIFIED OWNER PRIOR TO MODIFICATIONS.
6. PIPING OPENINGS THROUGH WALLS UNDER 4" ø SHALL BE CORE DRILLED BY THE RESPECTIVE TRADE CONTRACTOR. PIPE OPENINGS THROUGH WALLS OVER 4" ? SHALL BE CORE DRILLED AND/OR OTHERWISE SAW CUT BY THE G.C.
7. ALL HEADERS SHALL BE DOUBLE 2 x 12 UNLESS NOTED OTHERWISE ON DRAWINGS.
8. ALL OUTLETS TO BE LOCATED BY OWNER AT TIME OF CONSTRUCTION SHALL BE G.F.C.I. TYPE.
9. ALL WIRING SHALL FOLLOW THE MIN. REQUIREMENTS SET FORTH BY LOCAL, NATIONAL (N.E.C. 2014) AND STATE CODE (O.B.C.).
10. ALL LIGHT FIXTURES TYPES AND EXACT LOCATION WILL BE DETERMINED BY OWNER AT TIME OF CONSTRUCTION.
11. ALL FINISHES COLORS TO BE SELECTED BY OWNER AT TIME OF CONSTRUCTION.



Existing Aerial View N
NOT TO SCALE



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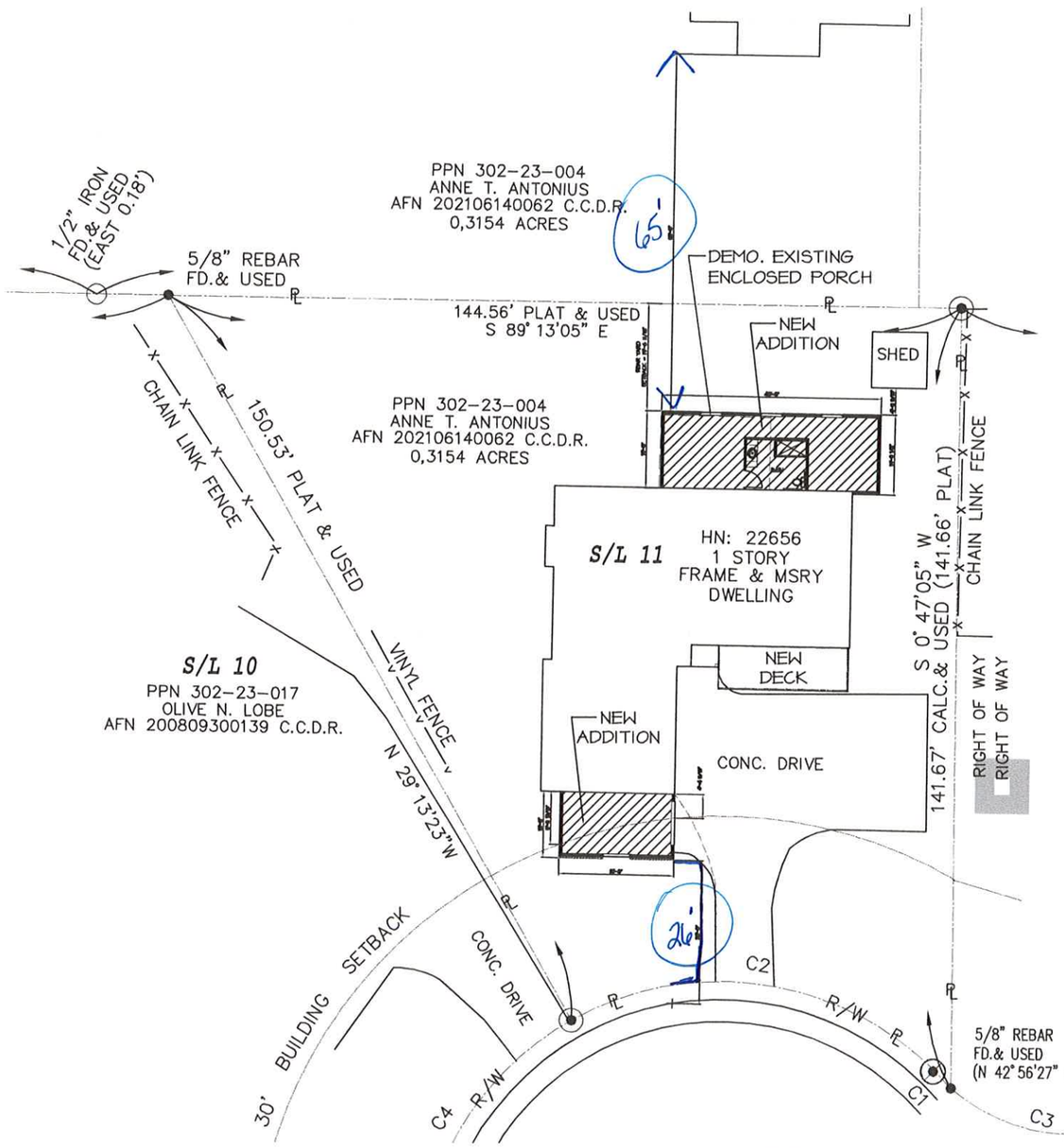

Site/Roof Plan NORTH
3/32"=1'-0"

Site Note

SITE AND BOUNDARY INFORMATION SHOWN ON THIS DRAWING WAS DERIVED FROM INFORMATION PROVIDED BY CUYAHOGA COUNTY PROPERTY SUMMARY REPORT & FORMAL SURVEY FOR PARCEL 302-23-004, 22656 PEACH TREE LANE, ROCKY RIVER, OHIO 44116 PROVIDED BY McSteen LAND SURVEYORS.

DWG. NO. C-1
DATE Oct. 31, 2022
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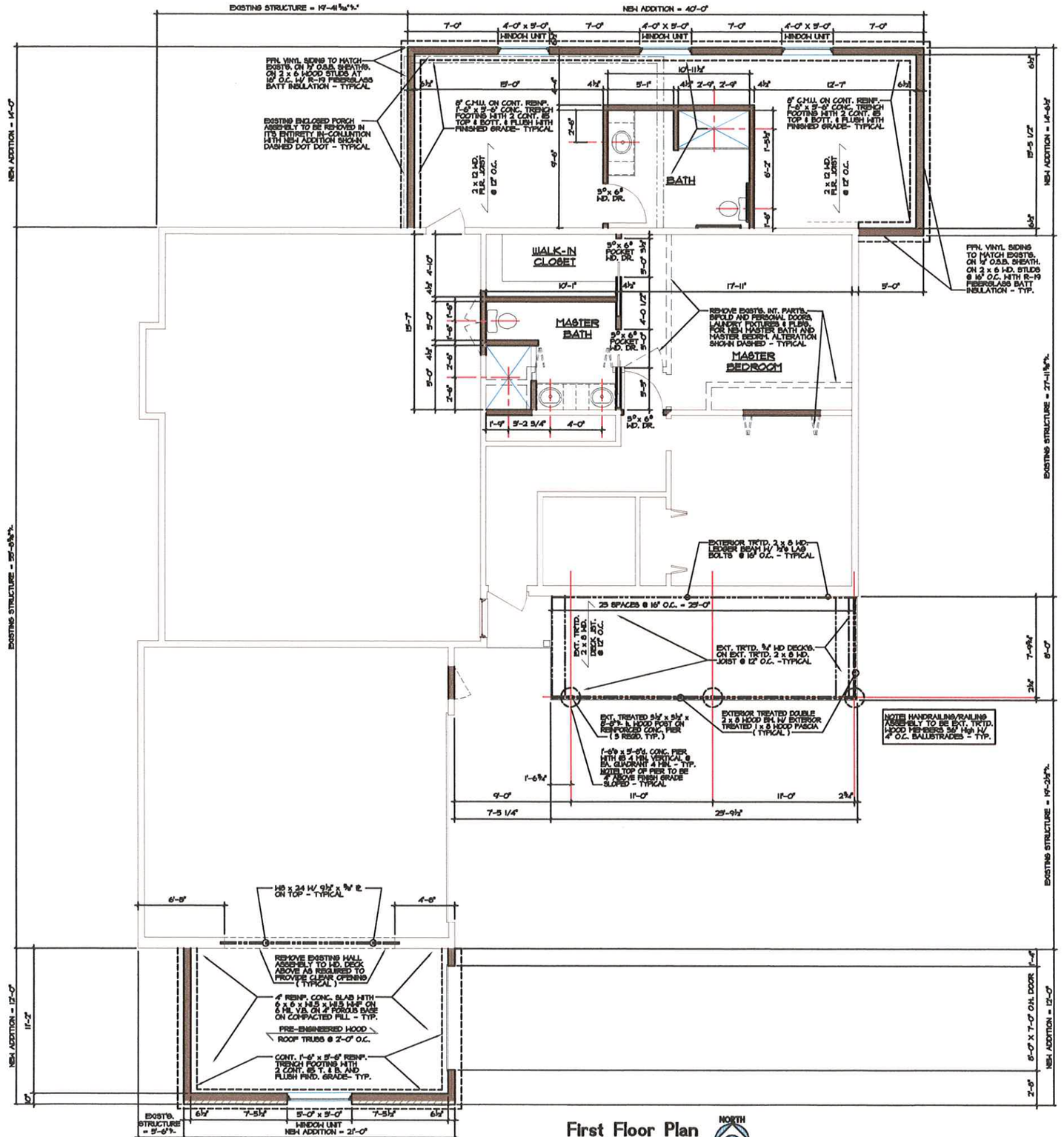

First Floor Plan

1/4"=1'-0"

NORTH

DWG. NO. A-1

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Structural Notes:

As Applicable

A. DESIGN LIVE LOADS
1. Slabs ———— 100 PSF
2. All Floors (Mezz.)— 100 PSF
3. Roofs ———— 30 PSF (25 LL. + 5 DL.)
4. Collateral ———— 10 PSF
5. Wind Load ———— 115 MPH
6. Earthquake-Zone I

B. DESIGN STRESSES

1. Soil Bearing Assumed/Soils Reports ————		2,000 PSF
2. Soil Bearing Pressure ————		2,500 PSF
@ Brown/Gray Silty Clay		
3. Soil Bearing Pressure————		3,500 PSF
@ Very stiff Brown/Gray Silty Clay		
4. Concrete (at 28 days)————	f'c =	4,000 PSI
5. Reinforcing Steel (A615 grade 60) ———	fy =	60,000 PSI
6. Structural Steel (A36) ————	fy =	36,000 PSI
7. Steel Joists (K Series) ————	fy =	50,000 PSI
8. Welded Wire Mesh (A-185)————	fs =	60,000 PSI
9. Steel Deck ————	fs =	20,000 PSI
10. Masonry - Solid Concrete Block ————		170 PSI
- Hollow Concrete Block ————		70 PSI
- Brick ————		150 PSI
11. Steel Deck————	fb =	33,000 PSI

C. CONCRETE CONSTRUCTION (unless otherwise noted)
1. Slabs on Ground - 6 x 6 W2.9 x W2.9 welded wire mesh reinforcing.
2. All concrete construction to be in accordance with the latest A.C.I. Code and A.C.I. Detailing Manual.
3. Reinforcing for slabs on ground shall be in the middle of the slab except as otherwise noted. Reinforcing shall be positively supported in this position and shall be maintained in this position during placing of concrete.
4. All epoxy-coated bars shall conform with ASTM A775 where required.
5. Cylinder test required by certified soil testing firm reporting on 7, 14, and 21 day test results.

D. FOUNDATIONS (unless otherwise noted)
1. Elevations given are to bottom of footings.
2. Bearing into Hard Brown grey clay - elevations given are approximate and shall be adjusted to suit field conditions.
3. Existing Foundations -
   a. Existing foundations as shown are approximate. Exact condition must be verified at time of construction.
   b. When new footings meet existing footings, they shall be stepped at ratio 2 to 1 slope (Two horizontal to one vertical).
4. Provide 2 - #5 top & botton continuous in all footings directly under masonry walls.
5. Footings shall be poured on hard dry earth. Open cuts shall not be exposed for more than 24 hours. No 'Cold Joints' permitted.
6. Compact all Fill to 100% of optimum laboratory density in accordance with ASTM D698 Standard Proctor Method. Place Fill in 8" layers and compact, with approved tamping/compacting equipment. A structural/geotechnical engineer shall be present for all concrete placement and approve same prior to pour.

E. STEEL CONSTRUCTION (unless otherwise noted)
1. Shop Connections - welded or bolted.
2. Field Connection - welded or friction type high strength bolts (A325SC) with hardened washers and threads included in the shear plane.
3. Shop drawings shall note type of electrode, size of all welds, and type and size of all bolts.
4. Painting - Provide shop primer coat (Grey).
5. Beams bearing on masonry shall have angle wall anchors.
6. Steel construction shall conform to the latest A.I.S.C. Specifications and Code of Standard Practice.
7. All shop and field welding shall be done by a certified welder.

F. MASONRY WALLS
1. Masonry walls shall be adequately braced to resist wind and other loads imposed during construction.
2. Provide a minimum of 8" deep x 16" wide solid masonry under bearing ends of all joists, slabs or beams.

G. ROOF OPENINGS
1. The contractor shall verify with the Heating/Ventilating Contractor the number, the size, and the location of all openings for his work and provide frames as detailed.

H. LINTELS
1. This schedule is for material not other wise shown or noted on drawings.

Span Limits	Angle Size
0' to 4'-0"	3-1/2" x 3-1/2" x 1/4"
4'-1" to 5'-6"	4" x 3-1/2" x 5/16"
5'-7" to 7'-6"	5" x 3-1/2" x 5/16"
7'-7" to 9'-6"	6" x 3-1/2" x 3/8"

   A. Provide one angle for each 4" of masonry wall thickness.
   B. Angles exposed to weather shall be 3/8" minimum thickness and hot-dip galvanized.
   C. Minimum bearing shall be 6" each end.

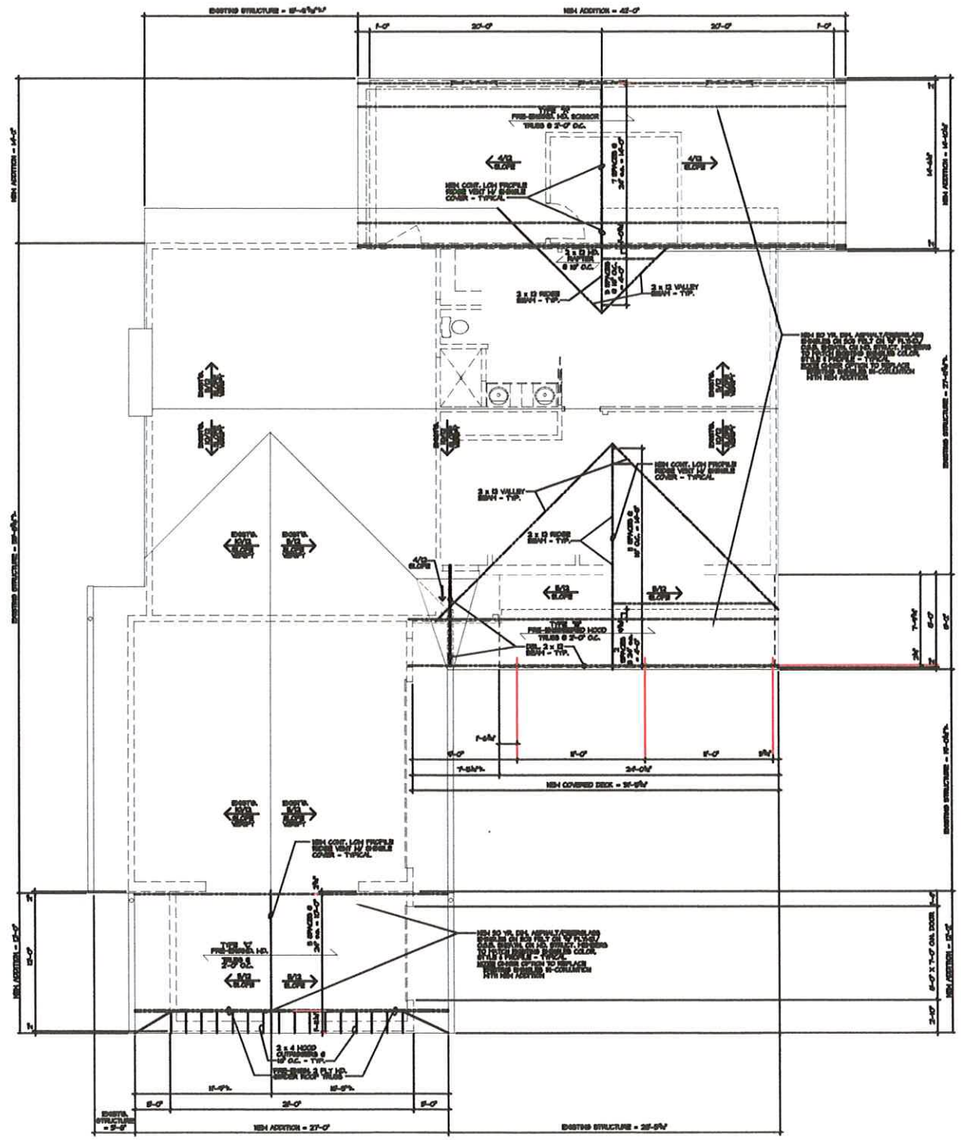
2. For 6" walls provide: Span to 4'-0" - WT 5 x 6.0
   to 5'-6" - WT 5 x 8.5
   to 9'-6" - 1-5" x 5/16" plate sten. Weld continuously both sides to 5" x 1/4" plate flange. (Make flange plate 3/8" thick at exterior openings).

I. STRUCTURAL WOOD (unless otherwise noted)
1. Premanufactured/ Preengineered Trusses
   Truss manufacturer shall supply the following information:
   a. Truss configuration, including span, pitch, loading diagram, and spacing of panel points.
   b. Loading requirements, shall be:

Top Chord	20 psf dead
	30 psf snow
Botton Chord	10 psf dead
	10 psf live

   c. Truss shall be design to take additional upper wall and roof loads including Snow Load for drifting and Mechanical Equipment. See Architectural and Mechanical Drawings for locations and weights.
   d. Species, stress grade, and nominal size of lumber used.
   e. Design live and dead loads including panel point loads and reactions.
   f. Manual or computer printout of axial forces and flexural stress plus design of member for combined loading as required.
   g. Joint and splice connection design details.
   h. Drawings and calculations stamped by a professional engineer registered in the State of Ohio.
   J. Design, fabrication and erection shall be in accord with the American Institute of Timber Construction (AITC) and attached in accord with Truss Plate Institue (TPI) recommendations, Fb = 1200, Deflection L/360, E= 1,760,00 psi

2. Roof Rafters - Douglas Fir with allowable design stresses: Fb = 1800, Deflection L/360, E= 1,760,00 psi
3. Ceiling Joist - Douglas Fir with allowable design stresses: Fb = 1800, Deflection L/360, E= 1,760,00 psi
4. All headers to be 2-2x12s unless noted otherwise on drawings.
5. All wood members shall conform to the latest standards and specifications adopted by the National Forest Products Association and the American Institute of Timber Construction



Roof/Framing Plan

3/16"=1'-0"

NORTH

DWG. NO. A-2

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Front Elevation

Front Right Side Elevation

Front Left Side Elevation

Left Side Elevation

Right Side Elevation

Existing Elevations

Not to Scale

DWG. NO. A-3

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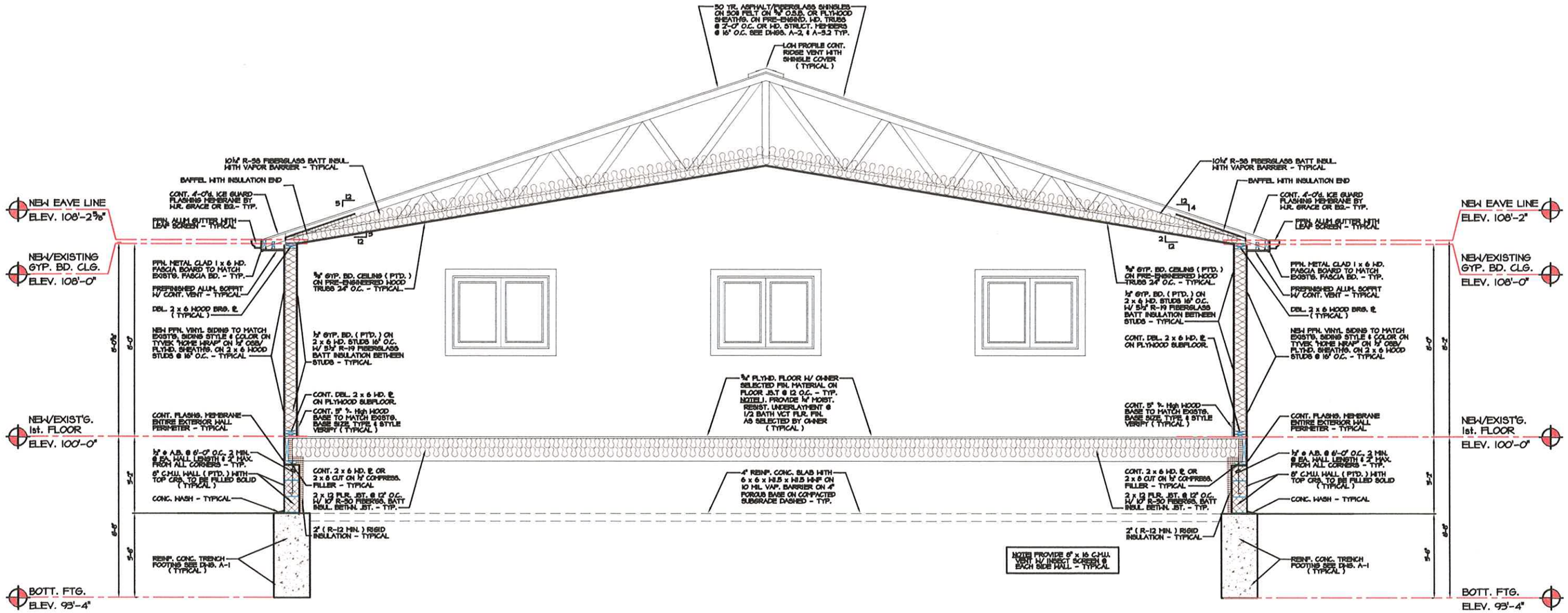

Wall/Cross Section

3/8"=1'-0"

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